

## APPENDIX D: A NON-LINEAR ELASTIC-PLASTIC MODEL FOR SAND

The model is isotropic, so it is convenient to formulate it in terms of mean and deviator stresses, which are also invariants.

### D1. Elastic Behaviour

The stress-strain law may be written in terms of the tangent bulk modulus,  $K_t$ , and the tangent shear modulus,  $G_t$ , as follows:

$$\begin{pmatrix} dp' \\ dq \end{pmatrix} = \begin{bmatrix} K_t & 0 \\ 0 & 3G_t \end{bmatrix} \begin{pmatrix} d\varepsilon_p \\ d\varepsilon_q \end{pmatrix} \quad (D-1)$$

where

$$\begin{aligned} dp' &= 1/3 [d\sigma'_1 + d\sigma'_2 + d\sigma'_3] \\ q &= \frac{1}{\sqrt{2}} \sqrt{(d\sigma'_1 - d\sigma'_2)^2 + (d\sigma'_2 - d\sigma'_3)^2 + (d\sigma'_3 - d\sigma'_1)^2} \\ d\varepsilon_p &= (d\varepsilon_1 + d\varepsilon_2 + d\varepsilon_3) \\ d\varepsilon_q &= \frac{\sqrt{2}}{3} \sqrt{(d\varepsilon_1 - d\varepsilon_2)^2 + (d\varepsilon_2 - d\varepsilon_3)^2 + (d\varepsilon_3 - d\varepsilon_1)^2} \end{aligned}$$

and  $d\sigma'_1$ ,  $d\sigma'_2$  and  $d\sigma'_3$  are increments of principal stresses, and  $d\varepsilon_1$ ,  $d\varepsilon_2$  and  $d\varepsilon_3$  are increments of principal strain.

### D2. Bulk Modulus, $K_t$

Following the suggestions of Naylor et al. (1981) and Lee and Salgado (2000), it is assumed that the tangent bulk modulus,  $K_t$ , is given by:

$$K_t = D_s (p')^{n_k} (p_a)^{(1-n_k)} \quad (D-2)$$

where  $p_a$  = atmospheric pressure

$D_s$  and  $n_k$  are material constants, with  $n_k$  usually having a value of about 0.5

Therefore  $K_t$  increases as a power law function of mean effective stress.

### D3. Shear Modulus, $G_t$

#### D3.1 Small strain shear modulus, $G_o$

Following Hardin and Black (1966), it is assumed that:

$$G_o = C_g \left( \frac{(e_g - e_o)^2}{1 + e_o} \right) (p_a)^{(1-n_g)} (p')^{n_g} \quad (D-3)$$

where  $C_g$ ,  $n_g$  and  $e_g$ , are material constants,

and  $e_o =$  initial void ratio

#### D3.2 Secant Shear Modulus, $G_s$

Lee and Salgado (2000) extended the work of Fahey and Carter (1993), by proposing an expression for the secant shear modulus, as:

$$\frac{G_s}{G_o} = \left( 1 - f \left( \frac{\sqrt{J_2} - \sqrt{J_{2o}}}{\sqrt{J_{2max}} - \sqrt{J_{2o}}} \right)^g \right) \left( \frac{I_1}{I_{1o}} \right)^{n_g} \quad (D-4)$$

where, the exponents,  $g$  and  $n_g$ , and the factor,  $f$ , are all material constants,

$$I_1 = 3p'$$

$$J_2 = 1/6 [(\sigma'_1 - \sigma'_2)^2 + (\sigma'_2 - \sigma'_3)^2 + (\sigma'_1 - \sigma'_3)^2]$$

For triaxial conditions,  $J_2 = 1/3 (q)^2$  (D-5)

In equation D-4,  $I_{1o}$  and  $J_{2o}$  are the values of the stress invariants at the commencement of monotonic loading.

If  $J_{2max}$  is the maximum value of the second stress invariant which is attainable at the current mean stress,  $J_{2max}$  can be found from the appropriate yield function,

e.g. if  $q = Mp'$ , then

$$J_{2max} = \frac{1}{3} q_{max}^2 = \frac{1}{3} (Mp')^2 \quad (D-6)$$

where  $M =$  current gradient of the failure envelope in  $q$ - $p'$  space

$p' =$  current mean stress

However it should be noted that other failure criteria (e.g. Mohr-Coulomb) could be used to determine  $J_{2 \max}$  for general stress conditions.

### D3.3 Tangent Shear Modulus, $G_t$

For conditions of simple shear ( $\tau$ - $\gamma$ ), Fahey and Carter (1993) demonstrated that

$$\frac{G_t}{G_o} = \frac{\left(\frac{G_s}{G_o}\right)^2}{\left[1 - f(1-g)\left(\frac{\tau}{\tau_{\max}}\right)^{fg}\right]} \quad (\text{D-7})$$

By analogy it is proposed that the generalised form of this equation should be

$$\frac{G_t}{G_o} = \frac{\left(\frac{G_s}{G_o}\right)^2}{\left[1 - f(1-g)\left(\frac{\sqrt{J_2} - \sqrt{J_{2o}}}{\sqrt{J_{2\max}} - \sqrt{J_{2o}}}\right)^g\right]} \quad (\text{D-8})$$

Equations D-2 and D-8 define the generalised forms of  $K_t$  and  $G_t$  to be substituted into the elastic stress-strain law, equation D-1.

#### Limit on $G_t$

It may also be necessary to place a limit on  $G_t$  as it reduces with increasing deviator stress. If a limit is placed, the following condition must be satisfied:

$$\left(\frac{G_t}{G_o}\right) \geq r_g$$

where  $r_g$  is a material parameter.

$r_g = 0$  corresponds to no limit on the reduction of  $G_t$  with deviator stress

$r_g = 1$  corresponds to no reduction in  $G_t$  with deviator stress

## D4. PLASTIC BEHAVIOUR

Refer to the basic model described in Appendix C.